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1) Introduction

A key theorem describing the evolution of all fluvial^[1] and deep marine^[2] sediment transport systems is that of accommodation as governed by erosion and deposition along the slope profile (Fig. 1). Erosion and deposition depend on if a flow is under- or overloaded, with the critical bypass criterion defined when net erosion/deposition is zero. This critical bypass criterion is used to define an equilibrium slope profile, towards which the system will evolve. Here this condition will be described in terms of the critical Shields bypass number (dimensional shear stress) for equilibrium flow.

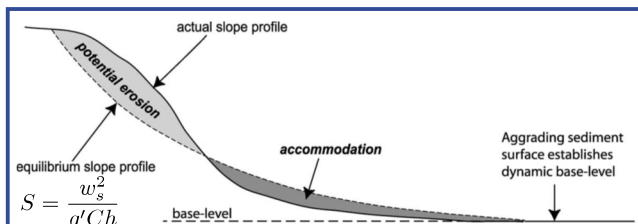
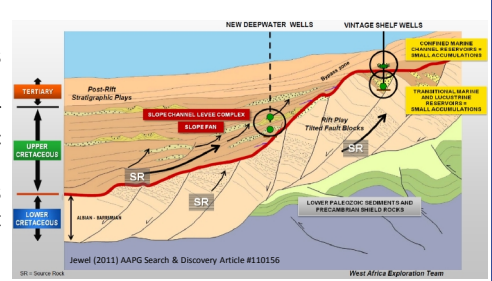


Figure 1. Accommodation space and turbidity current equilibrium slope profile as a function of sediment bypass; modified after [2].

Importance to E&P

The equilibrium profile determines locations of deposition and erosion and therefore the 1) distribution of reservoir sands and 2) position of stratigraphic pinchout traps produced by a transition from erosion/bypass to deposition. This for instance is critical to the abrupt margin play model (right) reliant on 'upslope stratigraphic pinchout traps.



2) Flow Bypass Models

Existing bypass criterion are summarized in Table 1. Here a depositional-erosional flux balance criterion (Fig. 2) is used as it enables analysis of the effect of grain size distribution on sediment bypass (§3). A comparison of different model bypass predictions and observations, using mean observational parameters $C=0.2\%$, $c_p=15^{-2}$ and $h=0.25$ (§4), is shown in Fig. 3.

Table 1. Summary of widely used empirical and theoretical bypass (equilibrium suspended load) sediment transport models.

Criterion Type	Source	Model	Notes
Type I	Rouse, 1937.	$u_* = w_s$	• Rouse-Bagnold stratification criterion.
	Bagnold, 1966.	$u_* = \frac{w_s}{0.085}$	• Monodisperse, average grainsize, model
	Ross et al., 1994.	$u_* = \frac{w_s}{0.085}$	• Le Roux recovers Rouse criterion for $w_s \ll u_*$
	Kubo et al., 2005.	$u_* = \frac{w_s}{0.085}$	$C_d \frac{\rho_s}{\rho_f} \frac{(\beta_s - 1)}{7003.7 \nu} u_*^3 + \dots \ll u_*$
Type II	Le Roux, 2005.	$\rho_s C_d u_*^3 = 7003.7 \frac{\rho_f}{(\beta_s - 1)} \frac{u_*^3}{\nu}$	$u_* = w_s + \frac{7003.7 \nu}{\rho_s C_d} u_*^3 + \dots \ll u_*$
	Rossinsky & Kuzman, 1950.	$C = \frac{0.024}{g'} \left(1 - \frac{\rho_f}{\rho_s}\right) \frac{u_*^3}{h w_s}$	• Flow power equilibrium concentration criterion.
	Celik & Rodi, 1991.	$C = \frac{0.034}{g'} \left(1 - \frac{\rho_f}{\rho_s}\right) \frac{u_*^3}{h w_s}$	• Concentration proportional to flow power.
	Wan & Wang, 1994.	$C = \frac{0.03}{g'} \left(\frac{u_*^3}{h w_s}\right)^{0.55}$	• Monodisperse, average grainsize, model.
Type IIIa,b	Dorrell & Hogg, 2011.	$c_i(Z_0) w_{si} = \frac{a_i}{C_m} \forall i$	• Equilibrium depositional-erosional flux criterion.
	Dorrell et al., 2013.	$E_i = \left\{ \begin{array}{l} m_i (u_*^2 - u_{*ci}^2)^{3/2} \\ 0 \end{array} \right. \begin{array}{l} u_* > u_{*ci} \\ u_* \leq u_{*ci} \end{array}$	• Type IIIa – monodisperse $N=1$
			• Type IIIb – polydisperse $N>1$
			• Recovers equilibrium concentration proportional to flow power for $u_* \ll u_{*ci}$
Parameters		$z = hZ$, $\xi_i(Z) = \left(\frac{Z_0}{1-Z_0} \frac{1-Z}{Z} \right)^{2m_i}$, $C_i = \frac{C_i \xi_i(Z)}{\int_{Z_0}^1 \xi_i(Z) dZ}$, $\sum_{i=1}^N C_i = C$, $\sum_{i=1}^N c_i = C$	
		$g' = g \left(\frac{\rho_s}{\rho_f} - 1 \right)$, $\theta = \frac{u_*^2}{g' d_{50}}$, $D_* = \left(\frac{g'}{u_*^2} \right)^{1/3} d_{50}$, $\tau_{*d}(Z_0) = Q u_* = Q c_i u_*^2$, $\phi = -\log_2 \left(\frac{d}{d_0} \right)$	

Figure 2. Type III depositional-erosional flow bypass schematic model for equilibrium suspended load sediment transport [3].

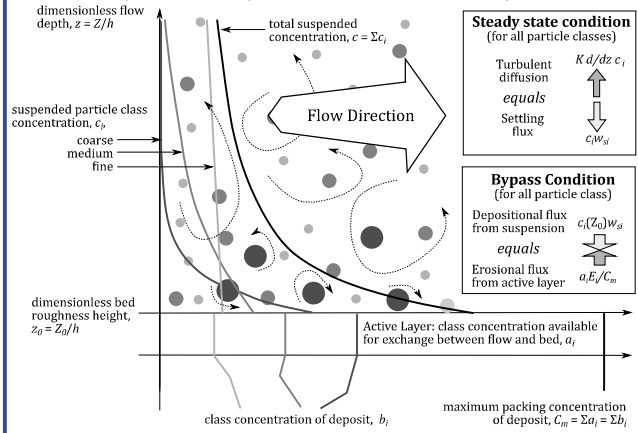
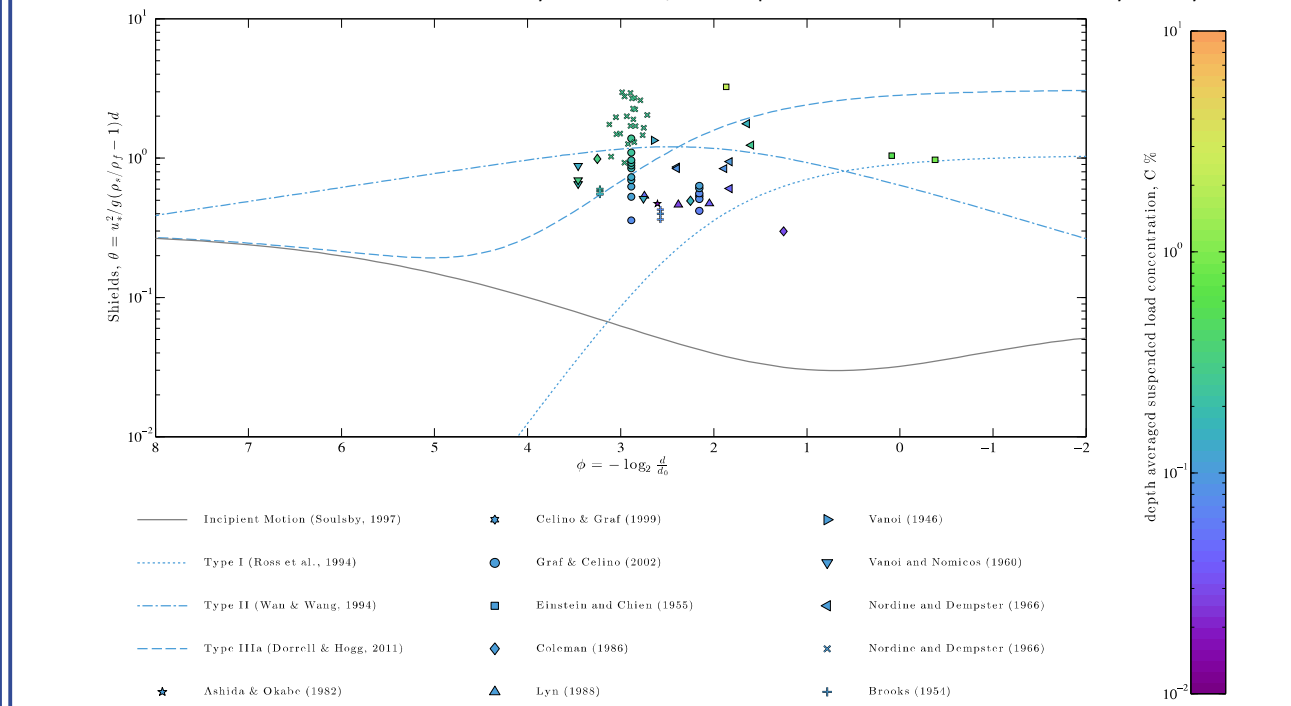


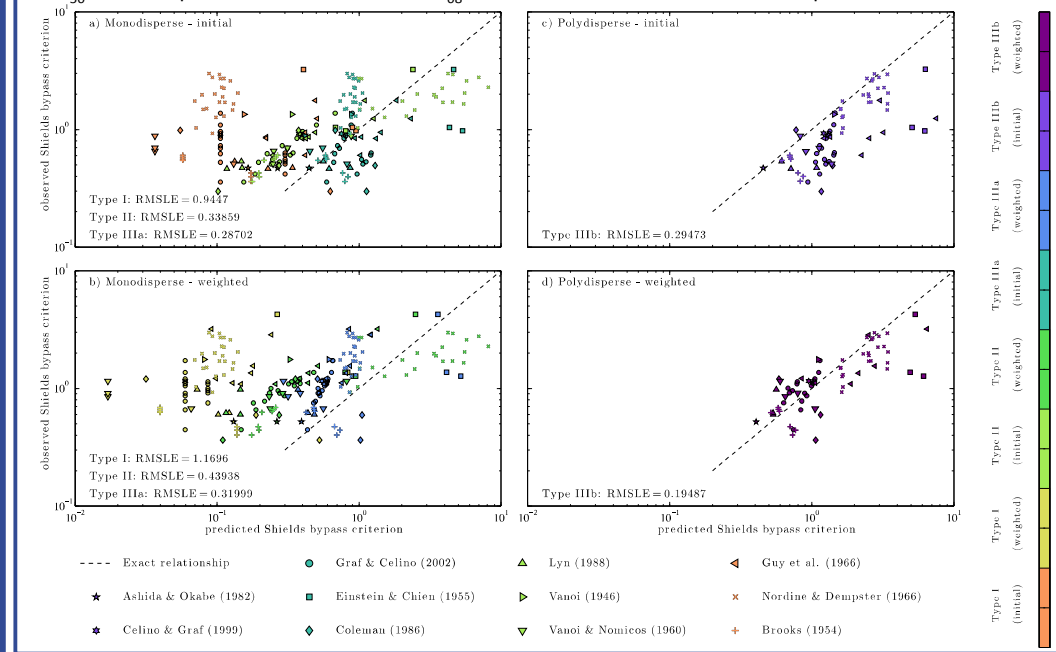
Figure 3. Shields bypass criterion predicted by the sediment transport models Type I (dotted curve), II (dash-dotted curve) and III (dashed curve), see Table 1. For a given suspended load and grain size, flows are net depositional below the criterion and net erosional above it. The Shields criterion for initiation of motion is denoted by a solid curve, whilst experimental observations are denoted by filled symbols.



4) Correlation with experimental and field based observations

The bypass models (Tab. 1) are compared against experimental and field observations of open channel flows (Fig 5). Results summarize use of both a monodisperse distribution, based on d_{50} , and polydisperse distributions, based on a skewed log-normal fit to initial and measured (or weighted) suspended grain size data.

Figure 5. (a)-(d) Correlation between predicted and observed Shields bypass criterion. For experiments weighted distributions reduce the mean grain size of suspended load material as a function of the initial distribution; here the d_{50} of the suspended load is taken as the d_{08} of the initial distribution based on empirical correlations.

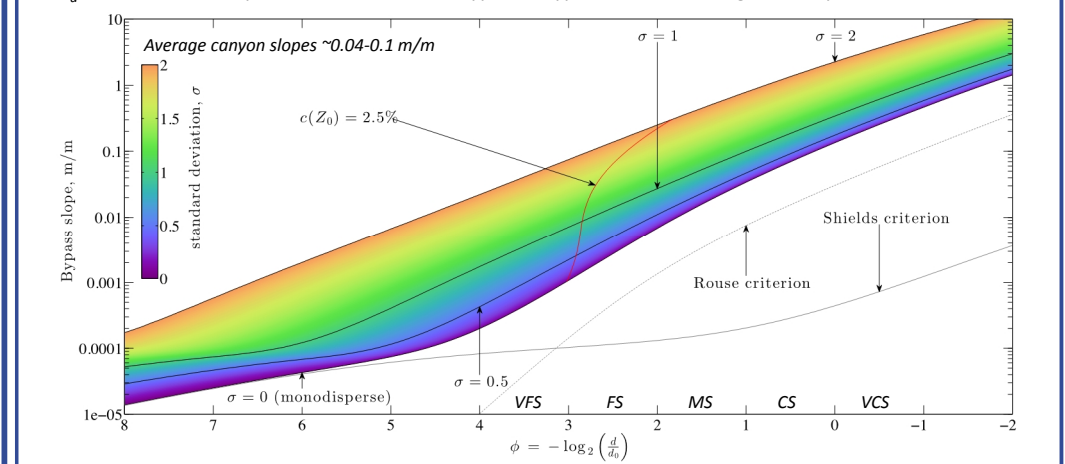


5) Implications for Turbidite Systems

For an equilibrium state turbidity current (with friction in balance with gravitational forces), where suspended load, flow height and drag coefficient are known, the Shields bypass criterion can be used to derive an equilibrium slope, S (Fig. 6). From [4] the equilibrium slope is defined in terms of bulk flow parameters:

$$S = \left(c_d + E \left(1 + \frac{1}{2} Ri \right) \right) Ri^{-1}. \quad (1)$$

Figure 6. The range of critical bypass slopes, derived from (1), for a 100m high turbidity current, where $C=0.1\%$ and $c_p=20^{-2}$. Flow velocity was derived from the Type IIIb bypass model with log-normally distributed sediment.



6) Conclusions

- New polydisperse bypass model provides best fit with experimental observations.
- Bypass Shields criterion and bypass slope varies with sediment distribution and flow load.
- Stratigraphic pinchout traps more likely to form in fine-grained and well-sorted systems.